ADDENDUM 4
Via E-Mail
DATE: December 09, 2025

Contract: 24-C-00032; TPD Howard Avenue Annex Building

Bidders on the above referenced project are hereby notified that the following addendum is made to the Contract Documents. BIDS TO BE SUBMITTED SHALL CONFORM TO THIS NOTICE.

Item 1 – Responses are provided for the following RFI's:

Section 10 73 16 – ALUMINUM CANOPIES

Response: Performance requirements and manufacturers found in this section.

Section 23 09 00 – INSTRUMENTATION AND CONTROL FOR HVAC; Part 2 – PRODUCTS; Section 2.2 CONTROL SYSTEM; Manufacturers.

Response: Remove items 1-4. REPLACE with 1. KMC. City of Tampa has KMC as a Single Source control manufacturer.

Sheet ID-101 INTERIOR FINISH PLAN

Response: Locations of roller shades are shown on this sheet.

Item 2 – Add attached STRUCTURAL ASSESMENT INSPECTION & EVALUATION REPORT

All other provisions of the Contract Documents and Specifications not in conflict with this Addendum shall remain in full force and effect. Questions are to be e-mailed to ContractAdministration@tampagov.net.

Jim Greiner

Jim Greiner, P.E., Contract Management Supervisor

MASTER CONSULTING ENGINEERS, INC.
STRUCTURAL CONSULTANTS

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STRUCTURAL ASSESMENT INSPECTION & EVALUATION REPORT

April 5, 2023

City Of Tampa
Construction Services
306 East Jackson Street

RE: COT Evidence Storage & Impound Lot
5005 N Howard Ave. Tampa, FL 33601
Structural Assessment of Existing Facility Building

FACILITY DESCRIPTION AND BACKGROUND INFORMATION

The existing facility building was built around the year 1983 as a warehouse and consists of a single-story building with 19.75' in height, and covers a footprint area of approximately 35,750 sf. It was used as a storage warehouse. The purpose of this evaluation is to verify compliance with current codes available to meet requirements for a risk category II structure.

STRUCTURAL DESCRIPTION

The building is currently classified as a non-essential building facility, Risk category II.

Based on existing dwg's issued by James B. Sullivan on September 1982 and observations made, structure was built as follows; building roof/diaphragm consist of a 9/16"C type corrugated deck, with 2" of light weight concrete topping and it's supported by 24H8 roof joists spaced at 48" O.C. Roof joists are carried by W steel beams, 6" Ø steel columns and 8" masonry bearing walls in the interior of the building, and masonry bearing wall, concrete tie beams and concrete columns spaced 16ft apart on the exterior of the building. Building is supported by shallow foundations that consist of continuous concrete strip footing to support masonry walls and spread footings for steel columns, foundations were designed not to exceed 2000psf of soil bearing capacity. Interior slab on grade was built as follows; 4" thick concrete slab and 5" thick slab on the loading duck areas. The lateral forces are resisted by exterior and interior masonry shear walls.

SITE VISIT ASSESMENT OBSERVATIONS

A representative of Masters Consulting Engineering, Inc. performed a site visit of the building noted above, the purpose of the site visit was to evaluate the existing conditions of the structure. The observation was limited to a visual observation of the visible elements, no testing, soil bearing capacity, scanning or destructive means were performed during the visit. In summary the overall structure appears to be in good conditions with minor observations as noted below (See appendix A for photographs)

- Cracks observed on interior and exterior slab on grade at a few locations.
- Masonry wall penetrations at some locations.
- Spalling on exterior slab stairs handrail support at loading dock access.
- Rust found at roof deck to tie beam connections.
- Moisture accumulation on exterior masonry walls at several areas.
- Observed loose bolts at exterior W beam to concrete column connection.

Recommendations: The above observations may not represent a significant risk to the structure, but it is our recommendation provide cleaning and remedial procedures. (See appendix B)

At the roof joist connections to top of concrete tie beams, it was not possible to observe the current condition or connection of the joist seat to embed plate, thus it was assumed that a field welded connection was provided.

No access to the roof was possible to evaluate the existing conditions, by aerial photographs, it can be observed some areas of moisture due to pounding water accumulation.

No visual access to verify roof deck diaphragm connections to roof joists or lap side fasteners, existing dwg's not provided with this information.

CURRENT CODE COMPLIANCE AND ANALYSIS

An analysis of the structural building was performed to confirm compliance with current codes for a Risk category II building using the following codes and manuals.

- The Florida Building Code (Seventh Edition) 2020, sections 506 and 605
- ACI Standard 318-14 Building code requirements for reinforced concrete.
- Building code requirements for Masonry Structures (TMS 402-16)
- AISC Specifications for the design, fabrication and erection of structural steel for buildings" 360-16 ASD
- ASCE 7-20 "Minimum Design Loads for Buildings and other Structures"
- SJI Steel Joist Institute.

The following wind data was used for the analysis: (See appendix "C")

Basic wind speed 141 mph (Ult.); 109mph (ASD)
Category risk II
Exposure C
Enclosed Classification
Internal Pressure Coefficient: ±0.18
Elevation: 27ft
Velocity Design Pressure: 38 psf

The following are the findings, recommendations and requirements:

Masonry wall were analyzed as bearing walls, and frame shear walls to resist lateral loads from diaphragm and were found to be sufficient to withstand forces and comply with minimum requirements. However, as a mean to transfer wind out of plane loads, some of the exterior walls were found insufficient to meet and resist wind out of plane loads in combination with sustained dead loads, due to no reinforcement used in block cells. Walls required to be reinforced with vertical #5 rebar at 48" O.C. Refer to appendix "D" for location of walls to be reinforced.

The steel frames were analyzed using the criteria of the AISC ASD, with the following materials:

W Beams, A36, Fy 36,000psi
HSS Columns pipes, A36, Fy 36,000psi
Bolts A325, Fu 105ksi
Anchor Bolts, F1554, Gr 36, Fy 36ksi
Plates and angles, A36, Fy 36ksi
Joists H series, 50,000psi

Steel beams and columns were found adequate to comply with current codes.

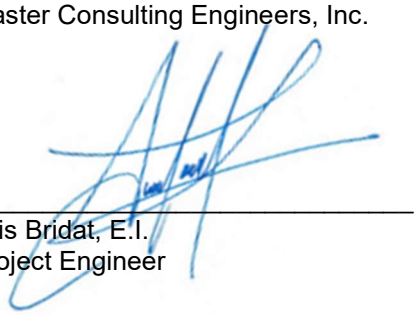
Roof joists were found adequate to sustain gravity loads and design and service level. However no horizontal or x bridging was observed at interior and exterior joists at first panel point. It is required to install a continuous bridging at first panel point at both ends. The minimum weld required to the joist seat and embed plate would be 3/16" x 3" at each joist.

Recommendations and requirements.

- Roof joists require to be provided with horizontal bridging at first panel point bottom chord as specify by the SJI.
- Exterior masonry walls as shown on Appendix “D” required to be reinforced with vertical #5 rebar at 48” O.C. See SSK-001 (Appendix “B)
- Recommend interior slab on grade crack repairs. See SSK-002 (Appendix “B”).

Should you have any questions or concerns, don’t hesitate to contact

Respectfully Submitted:
Master Consulting Engineers, Inc.



Luis Bridat, E.I.
Project Engineer

Appendix “A”

PHOTOGRAPHS:

Photo 1: Loading dock platform areas

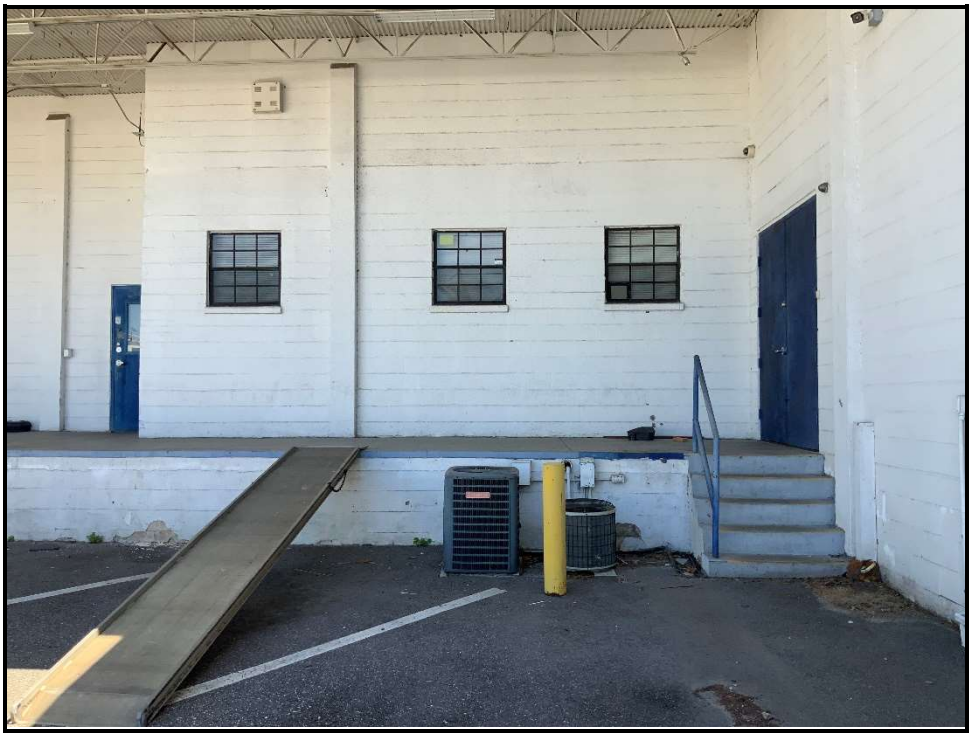


Photo 2: Loading dock platform and building access

PHOTOGRAPHS:

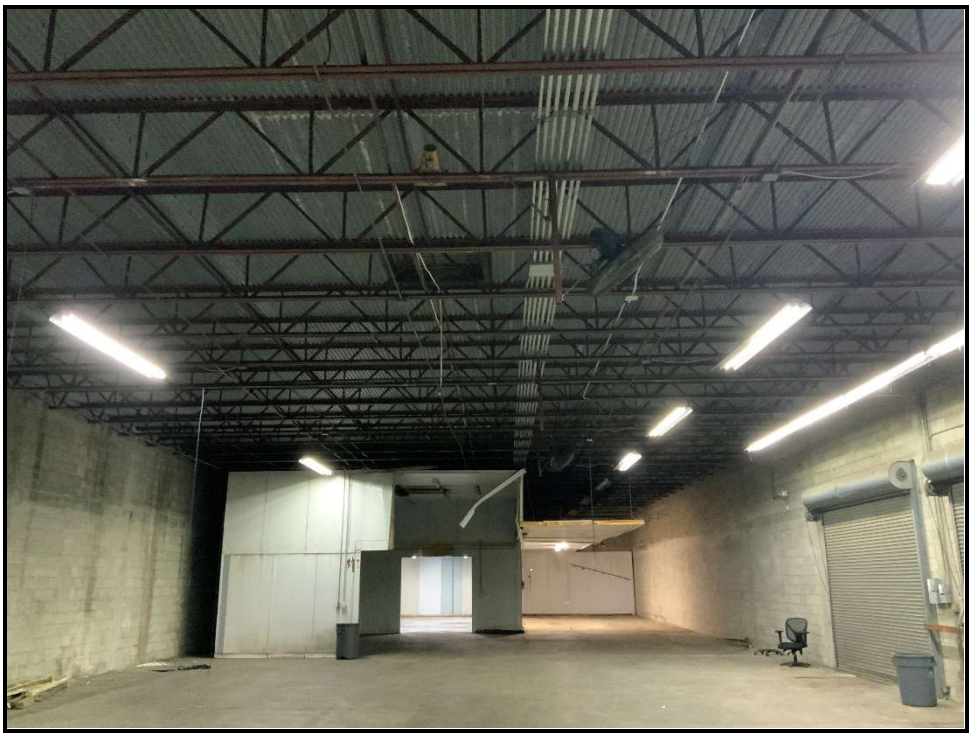


Photo 3: South west wing

Photo 4: South west wing typical roof framing and construction/expansion joint

PHOTOGRAPHS:

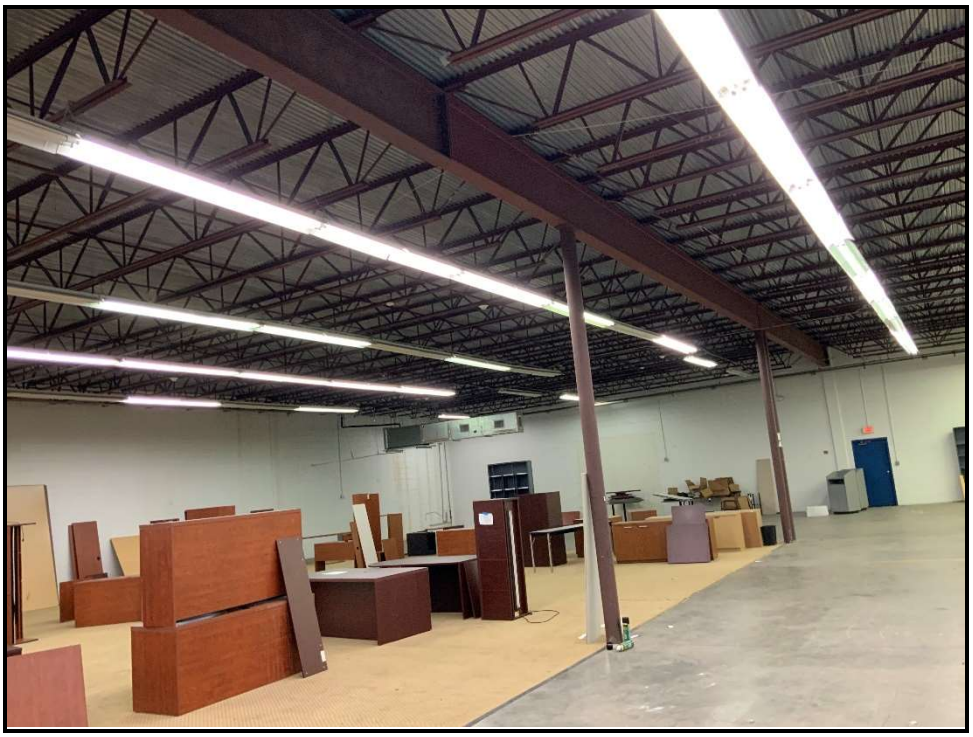


Photo 5: Steel frames placement. 6”Ø steel tube columns and W24 roof beams

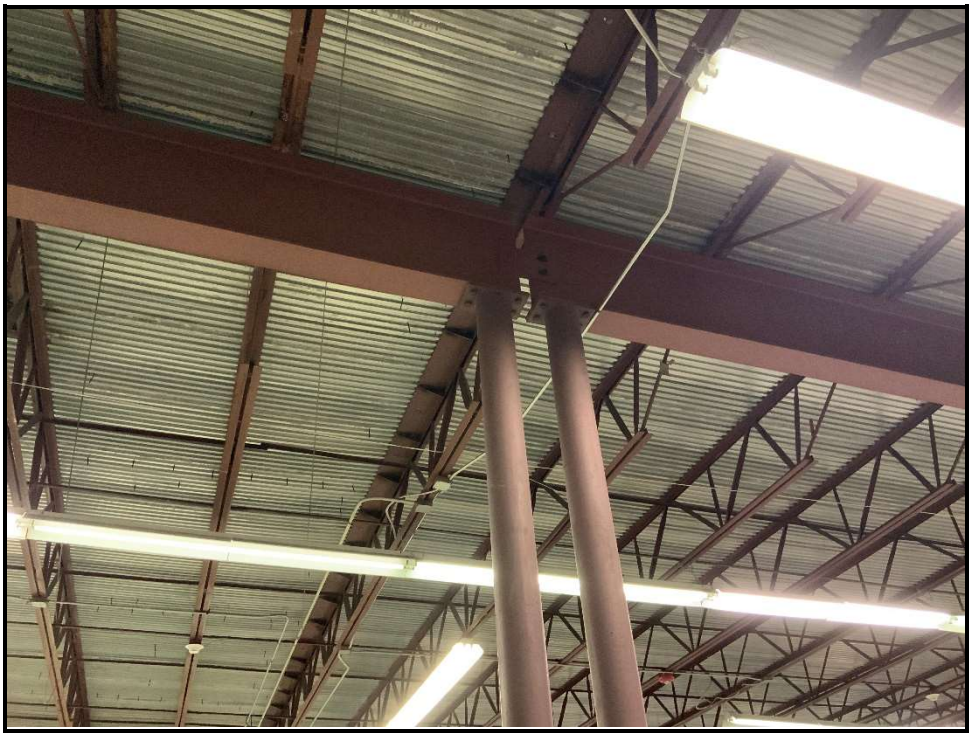


Photo 6: Steel frames at construction/expansion joint

PHOTOGRAPHS:

Photo 7: Typical interior bearing wall arrangement

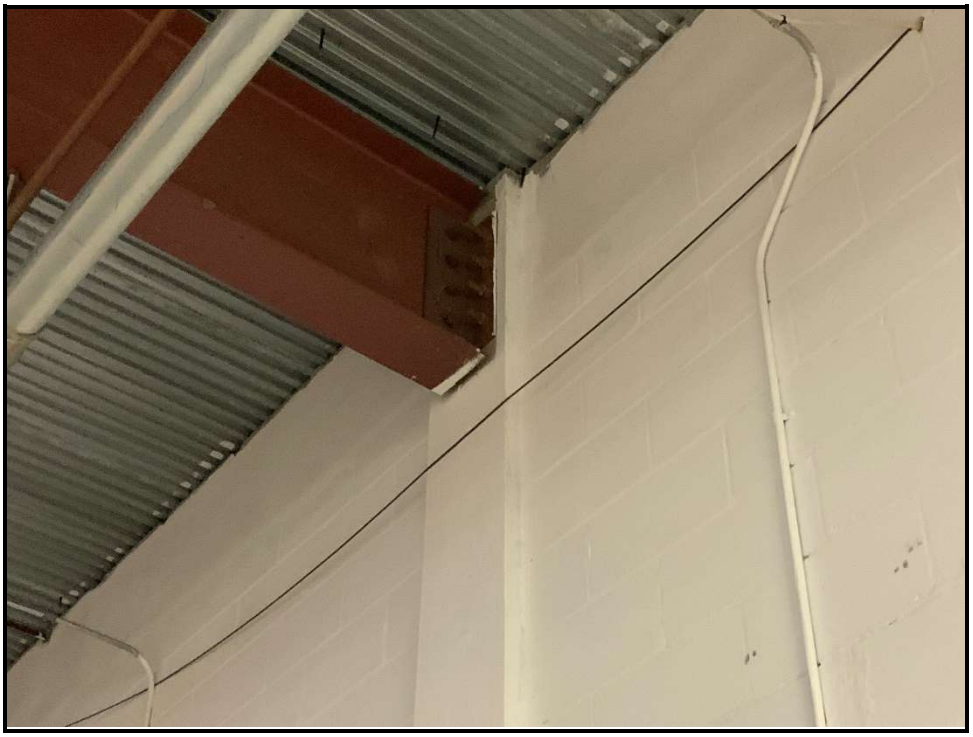


Photo 8: W24 Steel beam to concrete column connection

PHOTOGRAPHS:

Photo 9: Typical roof joist seat at concrete tie beam.

Photo 10: West wing north wall partial elevation

PHOTOGRAPHS:

Photo 11: West wall elevations.

Photo 12: South walls elevation

PHOTOGRAPHS:

Photo 13: East wing north wall elevation

Photo 14: Sign of licking and rust

PHOTOGRAPHS:

Photo 13: Concrete column spalling

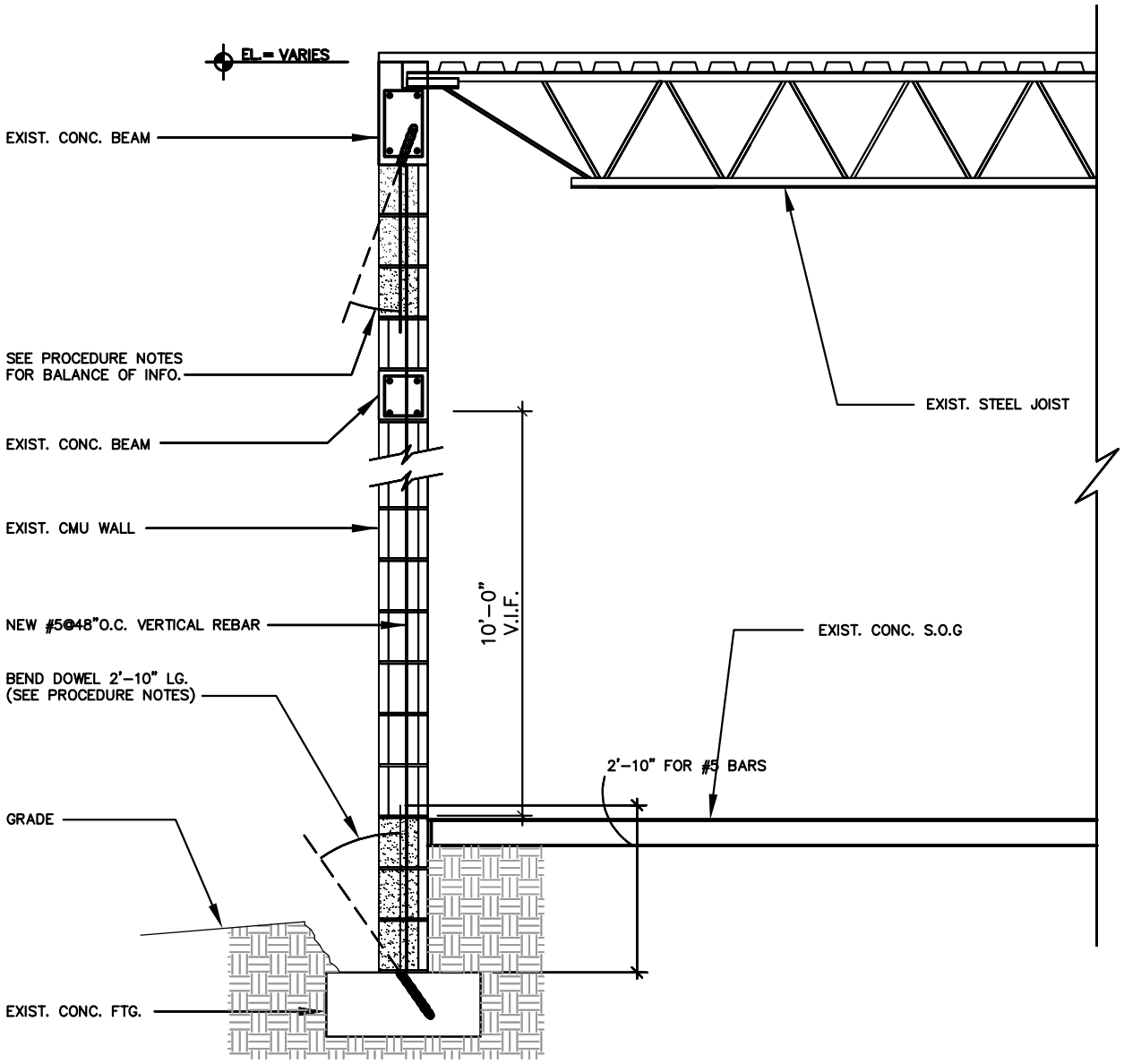
Photo 14: Concrete column spalling

PHOTOGRAPHS:

Photo 13: Typical slab on grade cracks

Photo 14: Loos bolt at frame connection

Appendix “B”



STRENGHTENING WALL DETAIL

SCALE: N/A

PROCEDURE NOTES FOR SECTION W2:

1. DIG EXTERIOR OF BUILDING TO EXPOSE EXISTING FOUNDATION.
2. LAYOUT THE ADDED BARS LOCATION AS SHOWN ON PLAN
3. BREAK THE EXTERIOR FACE OF CMU WALL TO INSTALL DOWEL (REBAR). DRILL & EPOXY DOWEL (REBAR) INTO EXISTING FOOTING (8" MIN. EMBED.) BEND BAR INTO THE CELL SPACING. (REPEAT PROCEDURE ON THE TOP UNDER TIE BEAM.
4. ON TOP OF THE WALL BREAK EXTERIOR FACE OF CMU WALL AS NEEDED TO INSTALL THE NEW ADDED VERTICAL REBAR.
5. FORM ALL OPENINGS DONE TO THE EXTERIOR WALL UP TO THE TOP ONE UNDER THE TIE BEAM. FILL CELLS WITH 3000 PSI CONCRETE.
6. AFTER CONCRETE REACH 75% OF ITS DESIGN STRENGTH, DRY PACK THE TOP CELL WITH NON-SHRINKAGE, NON-METALIC GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI.

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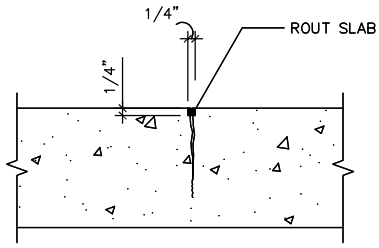
SUBJECT: EXTERIOR WALL BRACING
CLIENT: CITY OF TAMPA
PROJECT: COT TPD BUILDING ASSESMENT
SHEET NO. SSK-001 JOB NO: 3230.001.11
DATE: 4/10/2023 DES. BY: LB



CRACKS TYPE I
(20 MILLS TO 1/4")

SURFACE PREPARATION:

1. VEE NOTCH THE SURFACE OF THE CRACK WITH A MECHANICAL ROUTER OR HAND CHIPPING TOOL TO A MAXIMUM WIDTH OF A 1/4". REMOVE LOOSE DEBRIS.
2. REMOVE ALL DUST, LAITANCE, GREASE, CURING COMPOUND, WAXES, IMPREGNATION'S, FOREIGN PARTICLES, EFFLORESCENCE AND OTHER BOND INHIBITING MATERIALS FROM THE SURFACE BY MECHANICAL MEANS, I.E. – SANDBLASTING, HIGH PRESSURE WATER BLASTING, ETC.., AS APPROVED BY ENGINEER.
3. SEAL UNDERSIDE OF SLAB IF CRACKS REFLECT THROUGH WITH AN EPOXY GEL.

REPAIR PROCEDURE:

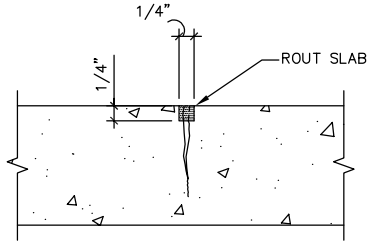
SIKA PRODUCTS:
1. POUR NEAT SIKADUR 35 HI–MOD LV EPOXY RESIN ADHESIVE INTO VEE NOTCHED CRACK. REPLENISH THE RESERVOIR WITH THE MIXED EPOXY RESIN ADHESIVE UNTIL CRACKS HAVE BEEN COMPLETELY FILLED.
2. IF PENETRATION OF ANY CRACK IS IMPOSSIBLE, CONSULT THE ENGINEER.

STO PRODUCTS:
1. POUR NEAT STO EPOXY BINDER CR633 INTO VEE NOTCHED CRACK. REPLENISH THE RESERVOIR WITH THE MIXED EPOXY RESIN ADHESIVE UNTIL CRACKS HAVE BEEN COMPLETELY FILLED.
2. IF PENETRATION OF ANY CRACK IS IMPOSSIBLE, CONSULT THE ENGINEER.

TYPICAL SECTION – CRACK TYPE 1 (20 MILLS TO 1/4")

SCALE: N/A

9–004



CRACKS TYPE II
(1/4" OR GREATER)

SURFACE PREPARATION:

1. VEE NOTCH THE SURFACE OF THE CRACK WITH A MECHANICAL ROUTER OR HAND CHIPPING TOOL TO A MAXIMUM WIDTH OF A 1/2". REMOVE LOOSE DEBRIS.
2. REMOVE ALL DUST, LAITANCE, GREASE, CURING COMPOUND, WAXES, IMPREGNATION'S, FOREIGN PARTICLES, EFFLORESCENCE AND OTHER BOND INHIBITING MATERIALS FROM THE SURFACE BY MECHANICAL MEANS, I.E. – SANDBLASTING, HIGH PRESSURE WATER BLASTING, ETC., AS APPROVED BY ENGINEER.
3. SEAL UNDERSIDE OF SLAB IF CRACKS REFLECT THROUGH.

REPAIR PROCEDURE:

SIKA PRODUCTS:
1. PRIOR TO PRODUCT APPLICATION SUBSTRATE SHOULD BE SATURATED SURFACE DRY (SSD) WITH NO STANDING WATER.
2. POUR NEAT SIKA TOP 111 INTO VEE NOTCHED CRACK. CONTINUE PLACEMENT UNTIL CRACK IS COMPLETELY FILLED.
3. IF PENETRATION OF ANY CRACK IS IMPOSSIBLE, CONSULT THE ENGINEER.

STO PRODUCTS:
1. FILL CRACK WITH OVEN–DRIED SAND PRIOR TO THE APPLICATION OF THE STO EPOXY BINDER.
2. POUR NEAT STO EPOXY BINDER CR633 INTO VEE NOTCHED CRACK. REPLENISH THE RESERVOIR WITH THE MIXED EPOXY RESIN ADHESIVE UNTIL CRACKS HAVE BEEN COMPLETELY FILLED.
3. IF PENETRATION OF ANY CRACK IS IMPOSSIBLE, CONSULT THE ENGINEER.

TYPICAL SECTION – CRACK TYPE II (1/4" OR GREATER)

SCALE: N/A

9–005

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EB: 8426

SUBJECT: CRACK REPAIR

CLIENT: CITY OF TAMPA

PROJECT: COT TPD BUILDING ASSESMENT

SHEET NO. SSK–002 JOB NO: 3230.001.11

DATE: 4/10/2023 DES. BY: LB

Appendix "C"

This is a beta release of the new ATC Hazards by Location website. Please contact us with feedback.

The ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

Hazards by Location

Search Information

Address: 5005 N Howard Ave, Tampa, FL 33603, USA

Coordinates: 27.9921893, -82.4809619

Elevation: 27 ft

Timestamp: 2023-03-30T19:56:24.140Z

Hazard Type: Wind

ASCE 7-16

MRI 10-Year ..... 79 mph

MRI 25-Year ..... 93 mph

MRI 50-Year ..... 105 mph

MRI 100-Year ..... 116 mph

Risk Category I ..... 131 mph

You are in a wind-borne debris region if you are also within 1 mile of the coastal mean high water line.

Risk Category II ..... 141 mph

You are in a wind-borne debris region.

Risk Category III ..... 150 mph

If the structure under consideration is a healthcare facility and you are also within 1 mile of the coastal mean high water line, you are in a wind-borne debris region. If other occupancy, use the Risk Category II basic wind speed contours to determine if you are in a wind-borne debris region.

Risk Category IV ..... 154 mph

You are in a wind-borne debris region.

ASCE 7-10

MRI 10-Year ..... 79 mph

MRI 25-Year ..... 94 mph

MRI 50-Year ..... 105 mph

MRI 100-Year ..... 116 mph

Risk Category I ..... 131 mph

You are in a wind-borne debris region if you are also within 1 mile of the coastal mean high water line.

Risk Category II ..... 141 mph

You are in a wind-borne debris region.

Risk Category III-IV ..... 150 mph

If the structure under consideration is a healthcare facility and you are also within 1 mile of the coastal mean high water line, you are in a wind-borne debris region. If other occupancy, use the Risk Category II basic wind speed contours to determine if you are in a wind-borne debris region.

ASCE 7-05

ASCE 7-05 Wind Speed ..... 116 mph

You are in a wind-borne debris region if you are also within 1 mile of the coastal mean high water line.

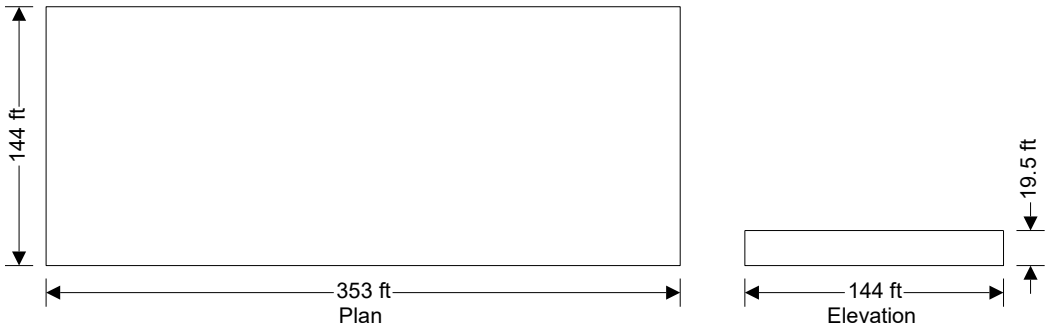
Tekla Tedds	Project				Job Ref.	
	Section				Sheet no./rev. 1	
	Calc. by L	Date 6/1/2023	Chk'd by	Date	App'd by	Date

WIND LOADING

In accordance with ASCE7-16

Using the envelope design method

Tedds calculation version 2.1.13



Building data

Type of roof	Flat
Length of building	b = **353.00** ft
Width of building	d = **144.00** ft
Height to eaves	H = **19.50** ft
Mean height	h = **19.50** ft
End zone width	a = max(min(0.1×min(b, d), 0.4×h), 0.04×min(b, d), 3ft) = **7.80** ft
Plan length of Zone 2/2E when GC_{pf} negative	L_{Z2} = min(0.5 × d, 2.5 × H) = **48.75** ft
Plan length of Zone 3/3E encroachment on zone 2	L_{Z3} = max(0 ft,0.5 × d - L_{Z2}) = **23.25** ft

General wind load requirements

Basic wind speed	V = **141.0** mph
Risk category	II
Velocity pressure exponent coef (Table 26.6-1)	K_d = **0.85**
Ground elevation above sea level	z_{gl} = **27** ft
Ground elevation factor	K_e = exp(-0.0000362 × z_{gl}/1ft) = **1.00**
Exposure category (cl 26.7.3)	C
Enclosure classification (cl.26.12)	Enclosed buildings
Internal pressure coef +ve (Table 26.13-1)	GC_{pi_p} = **0.18**
Internal pressure coef –ve (Table 26.13-1)	GC_{pi_n} = **-0.18**

Topography

Topography factor not significant	K_{zt} = 1.0

Velocity pressure

Velocity pressure coefficient (Table 26.10-1)	K_z = **0.90**
Velocity pressure	q_h = 0.00256 × K_z × K_{zt} × K_d × K_e × V^2 × 1psf/mph^2 = **38.7** psf

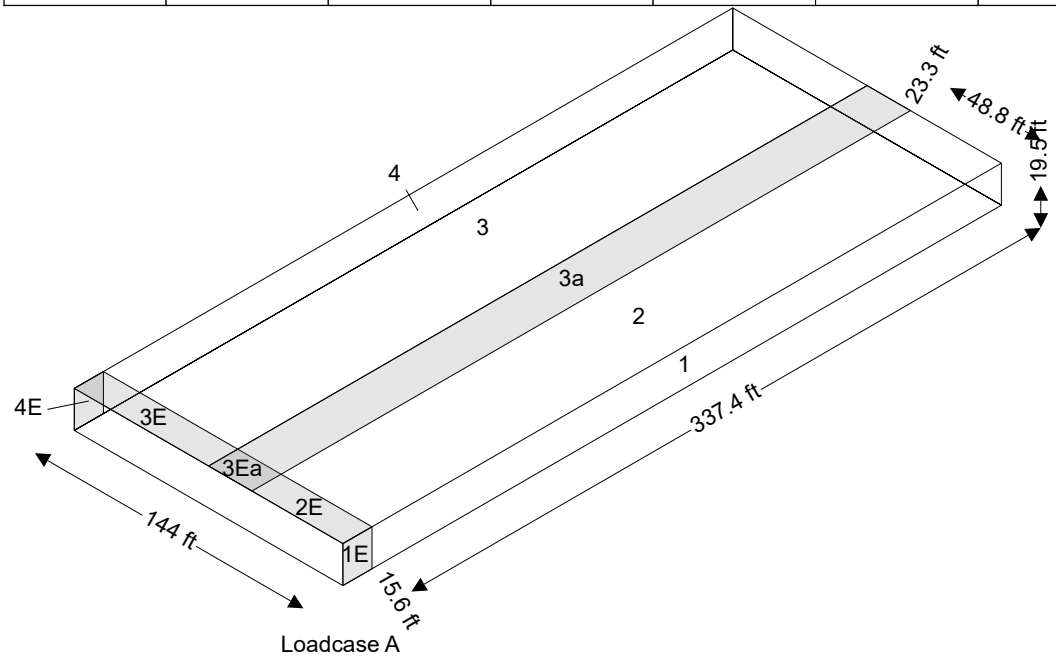
Design wind pressures

Design wind pressure equation $p = q_h \times [(GC_{pf}) - (GC_{pi})]$

Design wind pressures – Loadcase A

Zone	GC_{pf}	$p_{(+GCpi)}$ (psf)	$p_{(-GCpi)}$ (psf)	Area (ft^2)	$+F_{wi}$ (kips)	$-F_{wi}$ (kips)

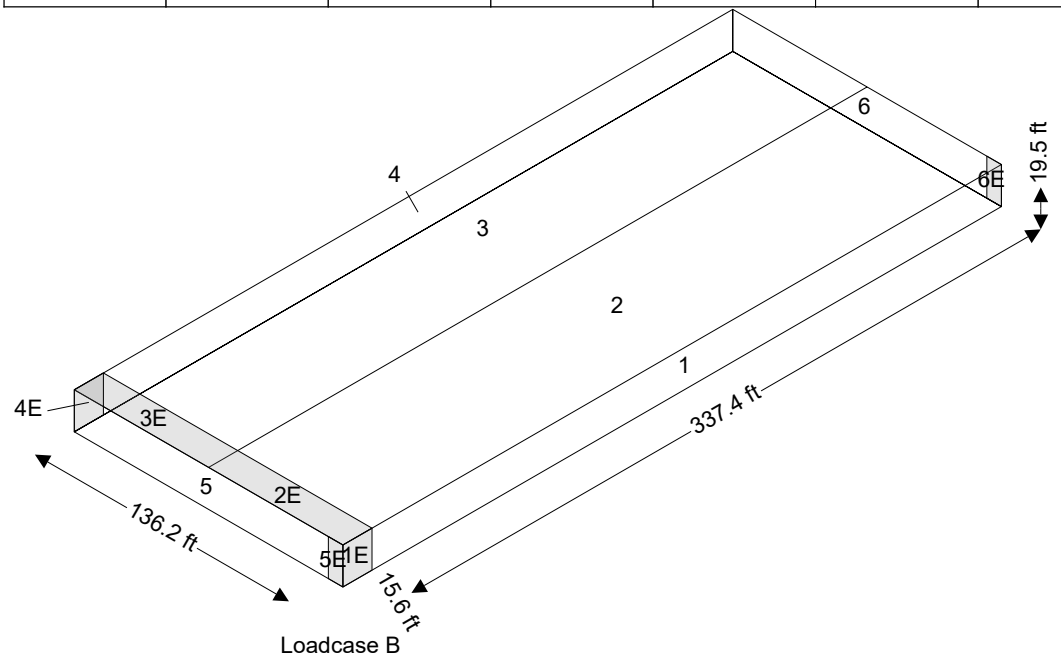
1	0.40	8.5	22.4	6579	56.0	147.6
2	-0.69	-33.7	-19.7	16448	-553.5	-324.5
3a	-0.37	-21.3	-7.3	7845	-166.9	-57.7
3	-0.37	-21.3	-7.3	24293	-516.8	-178.5
4	-0.29	-18.2	-4.3	6579	-119.6	-28.0
1E	0.61	16.6	30.6	304	5.1	9.3
2E	-1.07	-48.4	-34.4	760	-36.8	-26.2
3Ea	-0.53	-27.5	-13.5	363	-10.0	-4.9
3E	-0.53	-27.5	-13.5	1123	-30.8	-15.2
4E	-0.43	-23.6	-9.7	304	-7.2	-2.9



Design wind pressures – Loadcase B

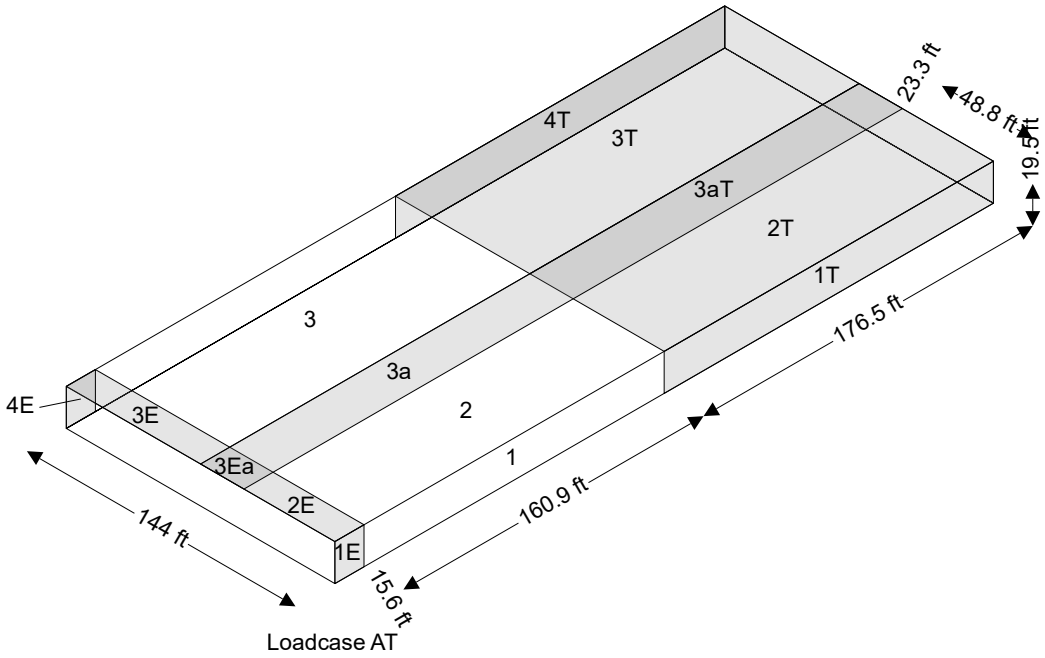
Zone	GC_{pf}	$p_{(+GCpi)}$ (psf)	$p_{(-GCpi)}$ (psf)	Area (ft^2)	$+F_{wi}$ (kips)	$-F_{wi}$ (kips)
1	-0.45	-24.4	-10.4	6579	-160.3	-68.7
2	-0.69	-33.7	-19.7	24293	-817.5	-479.2
3	-0.37	-21.3	-7.3	24293	-516.8	-178.5
4	-0.45	-24.4	-10.4	6579	-160.3	-68.7
5	0.40	8.5	22.4	2656	22.6	59.6
6	-0.29	-18.2	-4.3	2656	-48.3	-11.3
1E	-0.48	-25.5	-11.6	304	-7.8	-3.5
2E	-1.07	-48.4	-34.4	1123	-54.3	-38.7
3E	-0.53	-27.5	-13.5	1123	-30.8	-15.2

4E	-0.48	-25.5	-11.6	304	-7.8	-3.5
5E	0.61	16.6	30.6	152	2.5	4.6
6E	-0.43	-23.6	-9.7	152	-3.6	-1.5



Design wind pressures – Loadcase AT

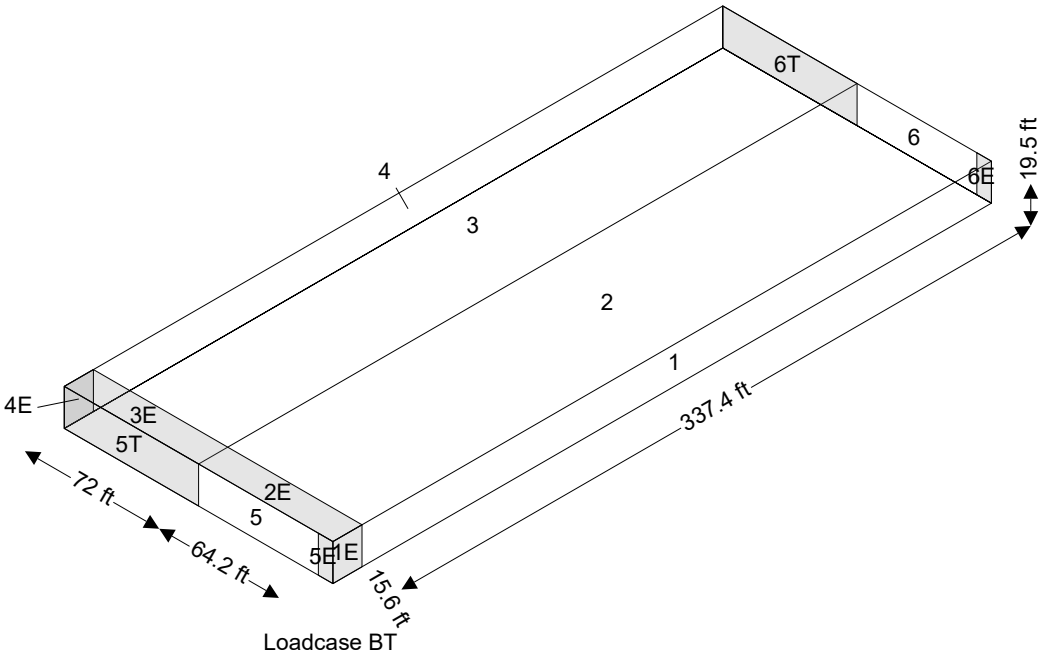
Zone	GC_{pf}	$p_{(+GCpi)}$ (psf)	$p_{(-GCpi)}$ (psf)	Area (ft^2)	$+F_{wi}$ (kips)	$-F_{wi}$ (kips)
1	0.40	8.5	22.4	3138	26.7	70.4
2	-0.69	-33.7	-19.7	7844	-264.0	-154.7
3a	-0.37	-21.3	-7.3	3741	-79.6	-27.5
3	-0.37	-21.3	-7.3	11585	-246.5	-85.1
4	-0.29	-18.2	-4.3	3138	-57.0	-13.3
1E	0.61	16.6	30.6	304	5.1	9.3
2E	-1.07	-48.4	-34.4	760	-36.8	-26.2
3Ea	-0.53	-27.5	-13.5	363	-10.0	-4.9
3E	-0.53	-27.5	-13.5	1123	-30.8	-15.2
4E	-0.43	-23.6	-9.7	304	-7.2	-2.9
1T	-	2.1	5.6	3442	7.3	19.3
2T	-	-8.4	-4.9	8604	-72.4	-42.4
3Ta	-0.09	-10.5	3.4	4104	-43.3	13.9
3T	-	-5.3	-1.8	12708	-67.6	-23.3
4T	-	-4.5	-1.1	3442	-15.6	-3.7



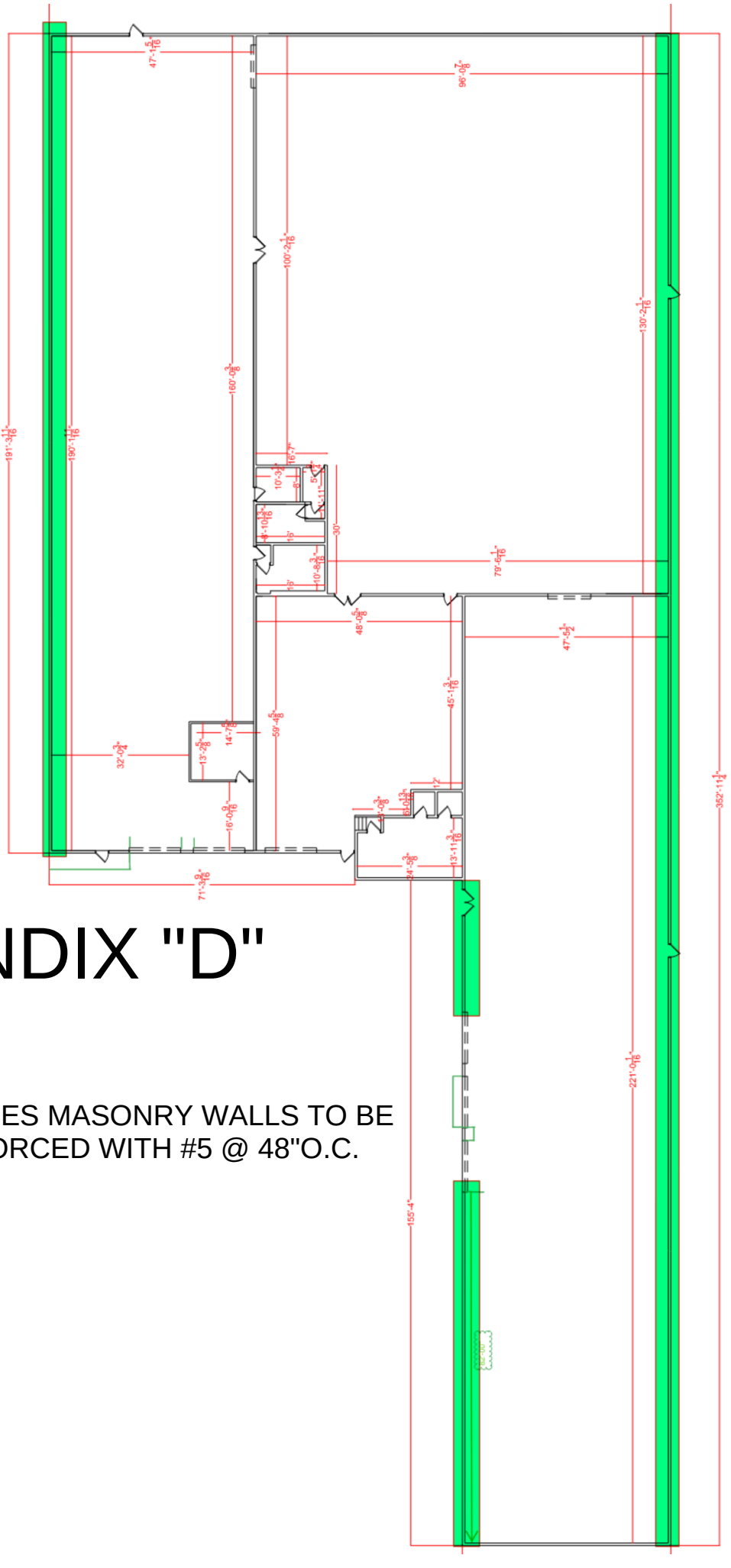
Design wind pressures – Loadcase BT

Zone	GC_{pf}	$p_{(+GCpi)}$ (psf)	$p_{(-GCpi)}$ (psf)	Area (ft^2)	$+F_{wi}$ (kips)	$-F_{wi}$ (kips)
1	-0.45	-24.4	-10.4	6579	-160.3	-68.7
2	-0.69	-33.7	-19.7	24293	-817.5	-479.2
3	-0.37	-21.3	-7.3	24293	-516.8	-178.5
4	-0.45	-24.4	-10.4	6579	-160.3	-68.7
5	0.40	8.5	22.4	1328	11.3	29.8
6	-0.29	-18.2	-4.3	1328	-24.1	-5.7
1E	-0.48	-25.5	-11.6	304	-7.8	-3.5
2E	-1.07	-48.4	-34.4	1123	-54.3	-38.7
3E	-0.53	-27.5	-13.5	1123	-30.8	-15.2
4E	-0.48	-25.5	-11.6	304	-7.8	-3.5
5E	0.61	16.6	30.6	76	1.3	2.3
6E	-0.43	-23.6	-9.7	152	-3.6	-1.5
5T	-	2.1	5.6	1404	3.0	7.9
6T	-	-4.5	-1.1	1404	-6.4	-1.5

Tekla Tedds	Project				Job Ref.	
	Section				Sheet no./rev. 5	
	Calc. by L	Date 6/1/2023	Chk'd by	Date	App'd by	Date



Loadcase BT



APPENDIX "D"

DENOTES MASONRY WALLS TO BE REINFORCED WITH #5 @ 48"O.C.