ADDENDUM 1
DATE: July 14, 2026

Contract: 26-C-00013; Hubert Avenue Park Improvements

Bidders on the above referenced project are hereby notified that the following addendum is made to the Contract Documents. BIDS TO BE SUBMITTED SHALL CONFORM TO THIS NOTICE.

Item 1: The bid date is hereby moved to July 28, 2026.

Item 2: Attached for reference is the Geotechnical Report.

The report for the project referenced comes from uncontrolled sources and does not conform to WCAG 2.1 at level AA. Individuals with disabilities seeking assistance with the individual project plans or specifications should e-mail Contractadministration@tampagov.net.

Item 3: Subsequent addendum will contain additional clarification.

All other provisions of the Contract Documents and Specifications not in conflict with this Addendum shall remain in full force and effect. Questions are to be e-mailed to ContractAdministration@tampagov.net.

Jim Greiner, P.E., Contract Management Supervisor

Professional Service Industries, Inc.
5801 Benjamin Center Drive, Suite 112
Phone: (813) 886-1075

Chris Anuszkiewicz, PLA, AICP, ISA
Professional Landscape Architect
Place Maker Design Studio, LLC
415 Plaza Drive
Dunedin, Florida 34698

RE: Addendum Report of Additional Exploration Services
Infiltration Testing and Evaluation of Foundation Plan
Proposed City of Tampa Park
NWC North Hubert Avenue & West Woodlawn Avenue
Tampa, Hillsborough County, Florida
PSI Project No. 0775-3740-A1

Dear Mr. Anuszkiewicz:

In accordance with your authorization requested activities, **Professional Service Industries, Inc. (PSI)** has performed additional services at the referenced site related to infiltration testing and evaluating the provided information regarding the proposed foundation plan.

To support the design and construction of proposed City of Tampa Park located at NWC North Hubert Avenue & West Woodlawn Avenue in Tampa, Florida, it was requested that a geotechnical exploration be performed. That work was completed and presented in our Geotechnical Exploration Report issued under PSI Project No. 0775-3740R1 on December 19, 2024. For additional information, please refer to our previously submitted report.

Infiltration Testing

After the submission of our December 2024 report, three (3) Double Ring Infiltrometer Tests were requested at the subject site (one per planned stormwater retention area). The requested DRI tests were collected on January 27, 2025. The test locations were identified in the field with a recreational grade hand-held GPS unit; therefore they should be considered approximate. The tests were conducted at approximate depths of 3 feet below the existing ground surface and in general accordance with applicable ASTM Standards. Presented below and attached are the results of the DRI tests and a figure presenting the approximate location of the tests.

Location	Test Depth (inches)	Soil Description (USCS)	Measured Infiltration Rate (in/hour)
DRI-1	36	SP/SP-SM	12½
DRI-2	36	SP/SP-SM	12¾
DRI-3	36	SP/SP-SM	7¾

Based on the obtained results and the observed subsurface conditions at the site, we recommend using a value of 25% for the sands fillable porosity. This value should be considered in the design of the stormwater management system to ensure adequate infiltration and storage capacity. An appropriate safety factor should be applied to infiltration and/or permeability rates for use in pond design by others. The stormwater management system should be designed and constructed in accordance with Water Management District requirements/criteria.

Evaluation of Foundation Plan

Based on correspondence with the project team, we understand that a portion of the proposed walking path is planned to be set on an elevated boardwalk system designed by a specialty firm. In the interest of protecting existing tree roots, the project team is contemplating designing the foundations for the elevated boardwalk with only a 4-inch embedment depth instead of the 18-inch embedment that was recommended in our report. We understand that the precast concrete footings subject to the reduced embedment would be 2'-3" square footings. Based on the provided information, we anticipate the maximum loading applied to an individual footing will be 12.5 kips. If any of this information is inconsistent with the planned project, we must be notified immediately so that we may amend the recommendations included in this document.

From a geotechnical perspective, the reduced embedment is allowable. However, it is our opinion that the reduced embedment will increase the risk of undermining the footings. If the project team desires to reduce this risk, structural fill or coarse aggregate (stone) may be imported and placed around the footings in controlled lifts (as discussed in Section 3.2 of our report) following footing construction. If this risk mitigation strategy is implemented, care should be taken to avoid damaging the newly constructed developments and any post-construction compaction activities should be performed with manual equipment.

Additionally, the project team should consider the potential risk of damage to the boardwalk if the nearby trees either die (and decompose) or continue growing (and potentially lift the structure). Such concerns are outside of our expertise as geotechnical engineers, and we offer this consideration exclusively as a service to our client. However, it should be noted that we do recommend the subgrade immediately beneath the footings consist of relatively clean sand (SP) or slightly silty sand (SP-SM) and no roots, as stated in our original report.

Closing

This report has been prepared for the exclusive use of Place Maker Design Studio, LLC and its consultants for the specific application to the proposed City of Tampa Park located at NWC North Hubert Avenue & West Woodlawn Avenue in Tampa, Florida.

We trust you will find this information suitable for your immediate needs. Please contact us if you have any questions regarding this report. Thank you for the opportunity to be of service to you.

Respectfully submitted,
PROFESSIONAL SERVICE INDUSTRIES, INC.

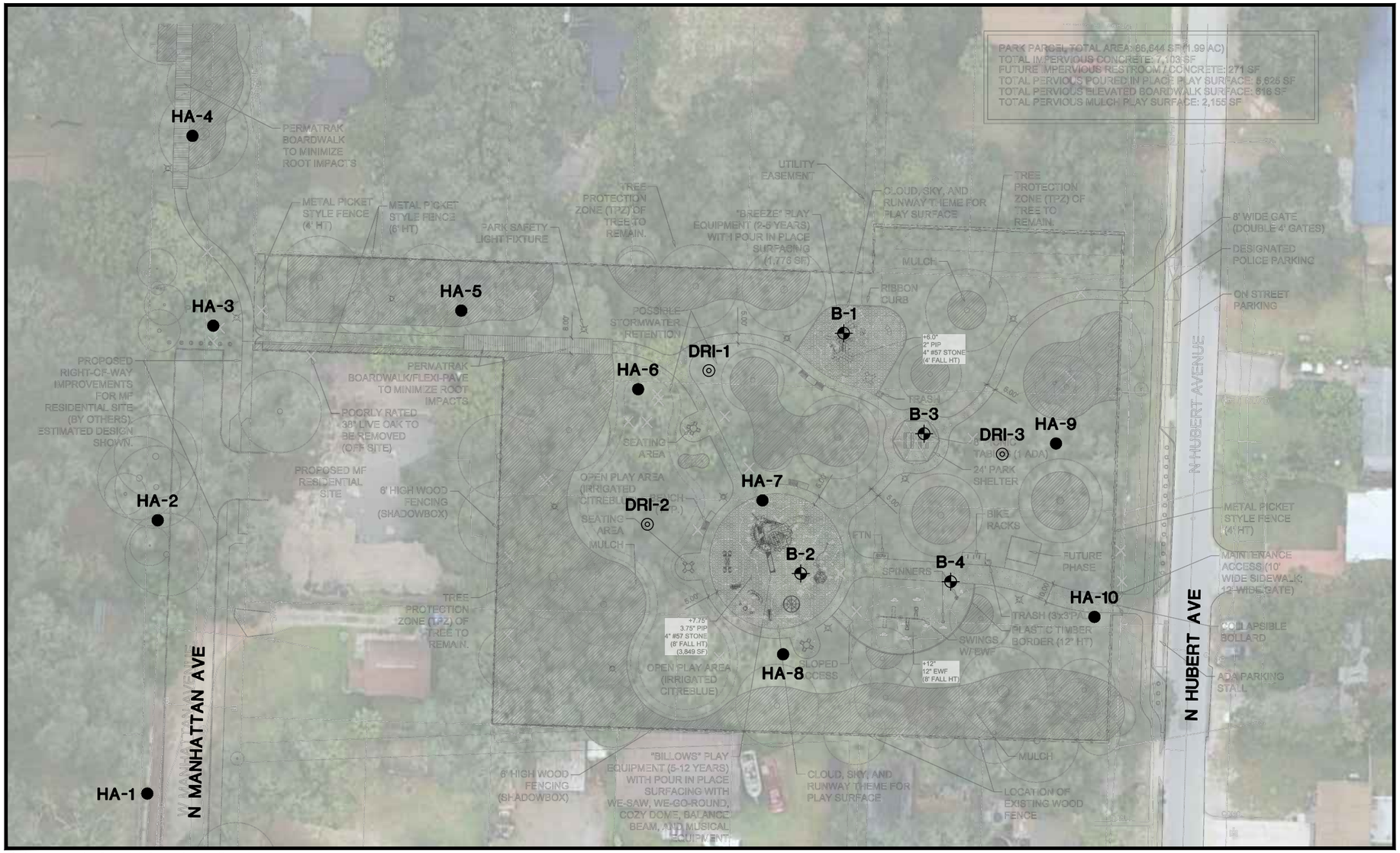
Isaac Valverde Lopez, E.I.
Staff Engineering Intern

Lloyd T. Lasher, Jr., P.E.
Principal Consultant
Florida License No. 56794

Cannon Gilbert, E.I.
Staff Engineering Intern

Courtland Alvies, P.E.
Geotechnical Services Department Manager

Attachments:
Sheet 1 – Boring Location Plan



Reference: Aerial obtained from Google

LEGEND

- Approximate SPT boring location
- Approximate Hand Auger boring location
- Approximate Double Ring Infiltrometer Test location

BORING LOCATION PLAN

0 60'

PLAN SCALE

intertek psi

DRAWN	DJG
CHECKED	CA
APPROVED	LTL
SCALE	NOTED

GEOTECHNICAL SERVICES

PROPOSED CITY OF TAMPA PARK

TAMPA, FLORIDA

DATE	PROJ. NO.	SHEET
NOV 24	07753740	SHEET 1

GEOTECHNICAL ENGINEERING SERVICES REPORT

For

Proposed City of Tampa Park
NWC North Hubert Avenue & West Woodlawn Avenue
Tampa, Florida

Prepared for

Place Maker Design Studio, LLC
415 Plaza Drive
Dunedin, Florida 34698

Prepared by

PROFESSIONAL SERVICE INDUSTRIES, INC.
5801 BENJAMIN CENTER DRIVE
SUITE 112
TAMPA, FLORIDA 33634
TELEPHONE (813) 886-1075
FAX (813) 249-4916

PSI PROJECT NO. 0775-3740R1

Originally Submitted December 13, 2024
Updated December 19, 2024

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phone: 813.886.1075
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intertek.com/building
psiusa.com
Engineering Certificate of Authorization 3684

Updated December 19, 2024

Chris Anuszkiewicz, PLA, AICP, ISA
Professional Landscape Architect
Place Maker Design Studio, LLC
415 Plaza Drive
Dunedin, Florida 34698

Re: Geotechnical Exploration Report
Proposed City of Tampa Park
NWC North Hubert Avenue & West Woodlawn Avenue
Tampa, Hillsborough County, Florida
PSI Project Number 0775-3740R1

Dear Mr. Anuszkiewicz:

Professional Service Industries, Inc. (PSI), an Intertek Company, is pleased to submit our geotechnical exploration report for the proposed City of Tampa Park to be located at the northwest corner of North Hubert Avenue and West Woodlawn Avenue in Tampa, Florida.

This report presents the results of our field exploration program and includes geotechnical engineering recommendations to guide design and construction of the project. This report has been updated to reflect topographic information regarding the proposed development site that was provided following the submission of the previous iteration of this report.

Should there be any questions, please do not hesitate to contact our office at (813) 886-1075. PSI would be pleased to continue providing geotechnical services and construction materials testing throughout the implementation of the project. We look forward to working with you and your organization on this and future projects.

Respectfully submitted,
PROFESSIONAL SERVICE INDUSTRIES, INC.

Isaac Valverde Lopez, E.I.
Staff Engineering Intern

Lloyd T. Lasher, Jr., P.E.
Principal Consultant
Florida PE No. 56794

Cannon Gilbert, E.I.
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Courtland Alvies, P.E.
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APPENDIX

1.0 PROJECT INFORMATION

1.1 PROJECT AUTHORIZATION

Professional Service Industries, Inc. (PSI), an Intertek Company, has completed a geotechnical exploration for the proposed City of Tampa Park to be located at the northwest corner of North Hubert Avenue and West Woodlawn Avenue in Tampa, Florida. This study has been performed in general accordance with our Geotechnical Engineering Services Proposal No. 0775-427868. Our services were authorized on October 24, 2024.

1.2 PROJECT DESCRIPTION

The request for proposal and project information was received by PSI through email correspondence from Mr. Chris Anuszkiewicz on May 29, 2024 and June 6, 2024. The request for proposal (RFP) included a conceptual site plan entitled "Hubert Avenue Park & Tampa Bay Blvd Linear Park Studies" drawn by Place Maker Design Studio, LLC. and dated May 31, 2024. Furthermore, boundary and topographic survey, "4106, 4108 & 4410 North Hubert Ave Tampa" prepared by Deuel & Associates, dated April 18, 2022, was also included. We understand a new recreational park is planned to be constructed at a moderately wooded lot located in the northwest corner of the intersection between North Hubert Avenue and West Woodlawn Avenue in Tampa, Florida. The subject lot resides between existing residential developments located to the north and south. The subject development will occupy approximately 2-acres of land. We understand the overall project includes planned portable bathrooms, gazebo, playground areas, proposed new trails and planned new parking lot areas (from the provided plans).

The proposed development consists of the design and construction of a small (less than 2,000 square foot) portable bathroom structure, gazebo structure of presently undefined footprint area (anticipated to be on the order of 2,500 square feet or less), two approximately 8,000 square foot playground areas, a series of unpaved trails and pathways, and new paved parking areas which will be located parallel to North Hubert Avenue.

Proposed grading and anticipated structural loads were not provided; however, based on previous experience with similar developments, we anticipate maximum cuts and/or fills on the order of 3 feet or less, maximum individual column loads of 50 kips, and maximum wall loads of 2 kips per lineal foot.

Should any of the above information or assumptions made by PSI be inconsistent with the planned construction, we request that you contact us immediately to allow us to make any necessary modifications to the recommendations on this report.

1.3 PURPOSE AND SCOPE OF SERVICES

The purpose of this study has been to obtain information on the subsurface soil and groundwater conditions within the proposed development. In order to achieve this objective, the following scope of services has been performed:

1. Four (4) total Standard Penetration Test (SPT) borings were performed for the project of which one boring was performed in the vicinity of the planned portable bathrooms, gazebo, and each playground area respectively. As requested, each boring was advanced to planned termination depths of 20-feet below the existing ground surface.

2. Ten (10) total manually excavated hand auger borings were performed along the proposed new trails and within the planned new parking lot areas of the project. As requested, each hand auger boring was advanced to a planned termination depth of up to 5-feet below the existing ground surface with the exception of two borings which could not be advanced due to dense root systems at wooded locations in the northwest corner of the site.

The borings were located in the field by estimating distances from existing features identified from the schematic site plan provided for this project; therefore, the Boring Location Plan presented on **Sheet 1** should be considered approximate.

The SPT borings were advanced using mud rotary drilling methods. In the SPT borings, samples were collected and SPT resistances were measured virtually continuously from 4 to 10 feet and on intervals of 5 feet thereafter to boring termination depths. The top 4 feet of the SPT borings were manually augured to confirm clearance for underground private utilities. Drilling and sampling techniques were accomplished in general accordance with ASTM standards. As called for by local regulations, the SPT borings were grouted upon completion.

Representative portions of each sample were returned to our Tampa, Florida office where they were visually classified by a geotechnical engineer. Classifications were performed in general accordance with the Unified Soil Classification System (USCS). Soil profiles were prepared for the boreholes indicating lithological materials encountered and any additional pertinent information.

At the conclusion of our field and laboratory work, the data was evaluated by an experienced geotechnical engineer to develop appropriate engineering recommendations.

2.0 SITE AND SUBSURFACE CONDITIONS

2.1 SITE LOCATION AND DESCRIPTION

The subject site is located at northwest corner of North Hubert Avenue and West Woodlawn Avenue intersection in Tampa, Florida. The subject site is currently vacant and moderately wooded. The subject lot resides between existing residential developments located to the north and south and will occupy approximately 2-acres of land.

2.2 SUBSURFACE CONDITIONS

Throughout the entire site, very loose to loose, relatively clean to slightly silty sands (SP and SP-SM), were encountered from near the existing ground surface to depths ranging from approximately 13 feet, underlain by medium dense clayey sand (SC) which frequently continued until boring termination depths were reached at depths of up to approximately 20 feet.

The soil profiles presented on **Sheet 2 and 3** include soil descriptions, approximate elevations and stratifications. Elevations were obtained using the information found within the provided survey plan, entitled, “Specific Purpose Survey 4106, 4108 & 4410 North Hubert Ave Tampa” drawn by Transystems and dated November 27, 2024. Variations may occur and should be expected between and away from the

widely spaced boring locations. The stratifications represent the approximate boundary between subsurface materials and the actual transition may be gradual.

2.3 GROUNDWATER INFORMATION

The groundwater table was found at elevations of approximately +23 to +26½ feet, NAVD 88, in the borings completed throughout the site. It should be noted that groundwater levels tend to fluctuate during periods of prolonged drought and extended rainfall and may be affected by man-made influences. In addition, a seasonal effect will also occur in which higher groundwater levels are normally recorded during the rainy season.

The estimated normal seasonal high groundwater levels presented herein are based on the observed soil stratigraphy, conditions observed in the borings, groundwater levels, USDA Soil Survey information, and our past experience in the project vicinity. Based on the borings completed to date, we estimate the normal seasonal high groundwater table elevation (SHWT) at the explored locations will occur at elevations of approximately +24½ to +28 feet, NAVD 88, which generally corresponds to depths of 2 to 3 feet below the existing ground surface, as originally estimated.

In general, the normal seasonal high groundwater level is not intended to define a limit or ensure that future seasonal fluctuations in groundwater levels will not exceed the estimated levels. Groundwater conditions will vary with environmental changes and seasonal conditions, such as the frequency and magnitude of rainfall patterns, as well as man-made influences, such as swales, drainage ponds, underdrains and areas of covered soil (buildings, paved parking lots, sidewalks, etc.).

3.0 EVALUATIONS AND RECOMMENDATIONS

3.1 GENERAL

The following recommendations have been developed on the basis of the previously described project characteristics and subsurface conditions encountered. If there is any change in the project criteria, including the location or orientation of any of the site elements, PSI should be allowed to review the plan to determine if additional fieldwork and/or any modifications to our recommendations will be required. Once final design plans and specifications are available, a general review by PSI is strongly recommended as a means to check that the evaluations made in preparation of this report are correct and that earthwork and foundation recommendations are properly interpreted and implemented.

3.2 SITE PREPARATION

1. Any vegetation, root mat, topsoil, and any other soft or unsuitable material in proposed construction areas should be removed. At a minimum, it is recommended that the clearing/stripping operations extend at least 10 feet beyond the proposed development perimeter, where possible. Material generated during stripping operations should be disposed of off-site in a proper manner. Initial site clearing and preparation work should be carried out under the observation of a representative of PSI's geotechnical engineer.

2. Following the clearing/stripping and rough grading operations, the exposed subgrade should be evaluated by a representative of PSI. The exposed subgrade soils should be compacted to at least 95 percent of the maximum dry density in accordance with ASTM D-1557, modified Proctor method. Any soft or unsuitable materials identified during the initial compaction should be removed and replaced with approved engineered fill.

3. Following subgrade preparation, development areas may be brought up to finished subgrade levels, if needed, using structural fill. The existing on-site fine sand to slightly silty fine sand (SP/SP-SM) can be used for structural fill provided it is free of organic or deleterious materials. Fill soils should be tested and approved by PSI prior to import and placement. Imported fill should consist of sand with less than 12 percent passing the No. 200 sieve, free of significant rocks/rubble, organics, clay, debris and other unsuitable material. Approved sand fill should be placed in loose lifts not exceeding 12 inches in thickness and should be compacted to at least 95 percent of the maximum dry density in accordance with ASTM D-1557, Modified Proctor method. Density tests to confirm compaction should be performed in each fill lift before the next lift is placed.

4. Prior to beginning compaction, soil moisture contents may need to be controlled in order to facilitate proper compaction. If additional moisture is necessary to achieve compaction objectives, then water should be applied in such a way that it will not cause erosion of the subgrade soils. A moisture content within the percentage range needed to achieve compaction (typically +/- 3 percent) is recommended prior to compaction of the natural ground and engineered fill.

A representative of PSI should be retained to provide full time, on-site observation of earthwork and excavation activities. It is important that PSI be retained to observe that the subsurface conditions are as we have discussed herein, and that fill placement is in accordance with our recommendations.

3.3 SHALLOW FOUNDATION RECOMMENDATIONS

With proper subgrade preparation, column footings and continuous wall foundations can be designed for a net allowable soil bearing capacity of 2,500 pounds per square foot, based on dead load plus design live load. Minimum dimensions of 36 inches for column footings and 18 inches for continuous footings should be used in foundation design to account for variable subsurface conditions which may be present, regardless of whether the maximum allowable foundation bearing pressures have been fully developed. Footings should bear at a depth of at least 18 inches below the final exterior grade. The embedment depth reduces the potential that adjacent excavations or erosion may undermine the exterior footings.

The foundation excavations should be observed by a representative of PSI prior to steel or concrete placement to confirm that the compacted materials are capable of supporting the design loads and are consistent with the materials discussed in this report. If the foundation excavations appear suitable as load bearing materials, the bottom of the foundation excavations should be compacted to at least 95 percent of the maximum dry density in accordance with ASTM D-1557, Modified Proctor method, for a minimum depth of one foot below the bottom of the footing depth, as determined by field density compaction tests. Soft or loose soil zones encountered at the bottom of the footing excavations should be removed and replaced with fill soils (as directed above), lean concrete, or dense graded compacted crushed stone (FDOT No. 57).

After opening, footing excavations should be observed and concrete placed as quickly as possible to avoid exposure of the footing bottoms to wetting and drying. Surface run-off water should be drained away from the excavations and not be allowed to pond. The foundation concrete should be placed promptly after the excavation is made.

3.4 SETTLEMENT

The settlement of shallow foundations supported on compacted native sand or engineered fill should occur rapidly after loading. Thus, the expected settlement should occur during construction as structural loads are imposed. Provided the recommended site preparation operations are properly performed, the total settlement of wall and isolated column footings from the applied loads is expected to be on the order of an inch or less. Maximum differential settlement across approximately 25 feet of continuous footing length is expected to be on the order of half the total settlement. Settlement of this magnitude is usually considered tolerable for most structures; however, the tolerance of the proposed structure to the predicted total and differential settlement should be confirmed by the project structural engineer.

3.5 FLOOR SLAB RECOMMENDATIONS

Slab-on-grade construction should be supported on soils compacted to at least 95 percent of the maximum dry density in accordance with ASTM D-1557, Modified Proctor method. We have assumed no extraordinary floor slab performance requirements such as very low allowable deflections or smoothness requirements are necessary. Any cuts that are made in the building pad for utility installation should be backfilled with clean granular materials that are compacted to at least 95 percent of the maximum dry density in accordance with ASTM D-1557, Modified Proctor method. Material to be placed within 12 inches of the bottom of the slab should have no single particle greater than 3 inches in size and should meet the requirements of approved structural fill.

The floor slab should be adequately reinforced to reduce the risk of cracking due to differential settlement. Floor slabs should not be rigidly connected to wall and column footings. An impervious membrane should be installed between the soil subgrade and bottom of floor slabs to be overlain with moisture sensitive coverings to avoid slab moisture problems. Floor slab design should conform to American Concrete Institute (ACI) design standards and practices.

3.6 PAVEMENT RECOMMENDATIONS

Grading information was not provided prior to the submission of this report but we anticipate relatively minor grade changes. We recommend site grading provide a minimum of 18 inches of separation between the estimated normal seasonal high groundwater table and the pavement basecourse for all pavement areas located throughout the site.

If a minimum separation of 18 inches is maintained between the bottom of the pavement base course and the estimated normal seasonal high groundwater table, pavement base materials may consist of limerock or crushed concrete. Base materials should meet FDOT requirements including being compacted to at least 98 percent of the maximum dry density in accordance with ASTM D-1557, modified Proctor Method, and a minimum LBR of 100 for limerock and minimum LBR of 150 for crushed concrete. Traffic should not be allowed on the subgrade as the base is placed to avoid rutting.

The following pavement sections should be considered minimal and are based on our experience with similar developments. Actual pavement sections should be determined by the project civil engineer based upon the anticipated traffic loading and expected pavement performance. Routine maintenance should be anticipated.

MINIMUM FLEXIBLE PAVEMENT RECOMMENDATIONS		
Material	**Minimum Thickness (inches)**	
	Light-Duty (Car Parking Areas)	**Medium-Duty (Drive and Entrance/Exit Areas)**
Type SP Asphaltic Concrete	1½	2½
Base Minimum LBR = 100 for Limerock and LBR = 150 for Crushed Concrete	6	8
Stabilized Subgrade Minimum LBR = 40	12	12

For heavy-duty uses, such as dumpster pads and areas to receive heavy truck traffic or frequent stopping and turning movements such as trash enclosures, we recommend the use of a rigid concrete pavement section. The concrete should have a minimum compressive strength of 4,000 psi at 28 days when tested in accordance with ASTM C-39. Based on our experience, a minimum concrete thickness of 7 inches should be utilized. The rigid pavement should be dowelled in accordance with FDOT Standard Index 305, as designed by the civil engineer. The upper 12 inches of subgrade immediately beneath the concrete pavement section should comprise AASHTO A-3 soils compacted to at least 98 percent of the maximum dry density in accordance with ASTM D-1557, modified Proctor method. Rigid pavement design recommendations are summarized in the following table.

MINIMUM RIGID PAVEMENT RECOMMENDATIONS	
Material	**Minimum Thickness (inches)**
Portland Cement (Concrete 4,000 psi minimum)	7
AASHTO A-3 Subgrade compacted to 98 percent of ASTM D-1557	12

All pavement materials and construction procedures should conform to the more stringent of Florida DOT or appropriate county/city requirements.

4.0 CONSTRUCTION CONSIDERATIONS

4.1 GENERAL

It is recommended that PSI be retained to provide observation and testing of construction activities involved in the foundation, earthwork, and related activities of this project. This will promote project continuity and will reduce the potential for misinterpretation of our recommendations.

4.2 DRAINAGE AND GROUNDWATER CONCERNS

Depending on finished grades and the time of year construction occurs, dewatering of this site may be necessary during some foundation and deeper utility construction. Dewatering should maintain the groundwater level at least 2 feet below the lowest excavation to facilitate compaction. We recommend that the dewatering system be designed, constructed, and maintained by an experienced dewatering contractor. The effects of the proposed dewatering system on adjacent properties or nearby structures should also be considered in the design. Dewatering systems should be designed and operated in accordance with current regulatory criteria.

Water should not be allowed to collect in the foundation excavations, on the floor slab areas, on the pavement surface, or on prepared subgrades of the construction area either during or after construction. Undercut or excavated areas should be sloped toward one corner to facilitate removal of any collected rainwater, groundwater, or surface runoff. Positive site drainage should be provided to reduce infiltration of surface water around the perimeter of the building and beneath the floor slabs. The grades should be sloped away from the building and surface drainage should be collected and discharged such that water is not permitted to infiltrate the backfill and floor slab areas of the building. In pavement areas, standing water on the pavement surface or within the base course layer can lead to softening of the subgrade and other problems related to the deterioration of the pavement.

4.3 UTILITY EXCAVATIONS

Utility trenches may be required for this project. We recommend all utility trench backfill comply to the gradation and requirements of engineered fill, as described above and be placed and compacted to the recommendations contained in the site preparation section of this report. Engineered fill should be compacted to at least 95 percent of the maximum dry density in accordance with ASTM D-1557, modified Proctor method.

4.4 EXCAVATIONS

In Federal Register, Volume 54, No. 209 (October 1989), the United States Department of Labor, Occupational Safety and Health Administration (OSHA) amended its “Construction Standards for Excavations, 29 CFR, Part 1926, Subpart P”. This document was issued to better ensure the safety of workmen entering trenches or excavations. It is mandated by this federal regulation that excavations, whether they be utility trenches, basement excavations or footing excavations, be constructed in accordance with current OSHA guidelines. It is our understanding that these regulations are strictly enforced and if they are not closely followed, the owner and the contractor could be liable for substantial penalties.

The contractor is solely responsible for designing and constructing stable, temporary excavations and should shore, slope, or bench the sides of the excavations as required to maintain stability of both the excavation sides and bottom. The contractors "responsible person", as defined in 29 CFR, Part 1926, should evaluate the soil exposed in the excavations as part of the contractor's safety procedures. In no case should slope height, slope inclination, or excavation depth, including utility trench excavation depth, exceed those specified in all local, state, and federal safety regulations.

We are providing this information solely as a service to our client. PSI does not assume responsibility for construction site safety or the contractor's or other party's compliance with local, state, and federal safety or other regulations. It is the policy of PSI not to provide recommendations regarding temporary slopes during construction which is the sole responsibility of the contractor as indicated above.

5.0 REPORT LIMITATIONS

The Geotechnical Engineer warrants that the findings, recommendations, specifications or professional advice contained herein have been made in accordance with generally accepted professional geotechnical engineering practices in the local area. No other warranties are implied or expressed.

The services provided were conventional in nature and did not include any special services that may lessen the risk of conditions that can contribute to moisture, mold or other microbial contaminant growth in buildings. You may be aware that mold is abundant throughout nature and is comprised of a wide variety of microscopic fungi. Due to its nature, the potential for mold infestations cannot be completely eliminated.

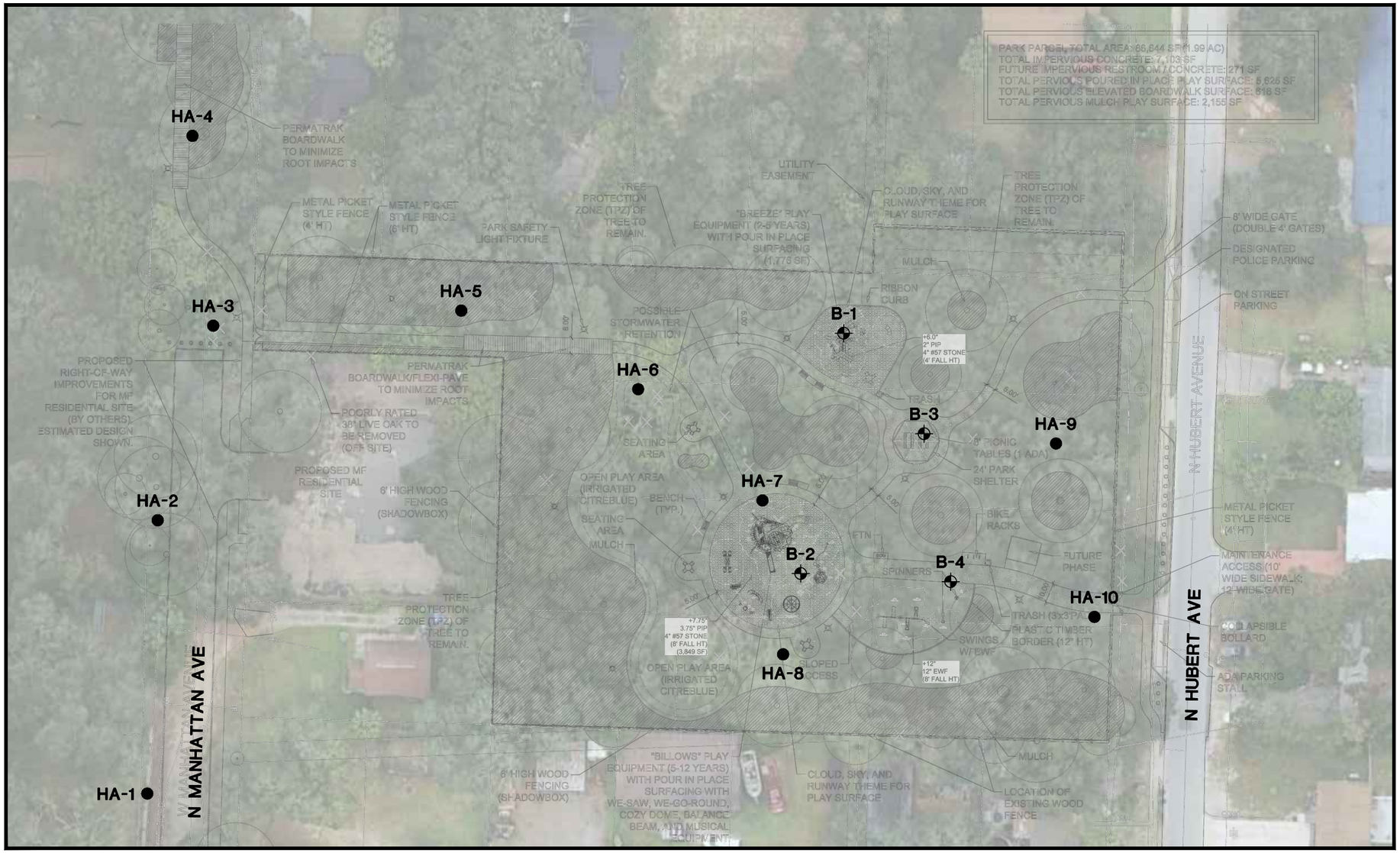
The scope of services also does not include an environmental assessment for determining the presence or absence of wetlands, or hazardous or toxic materials in the soil, bedrock, surface water, groundwater, or air on or below, or around this site. Any statements in this report or on the boring logs regarding odors, colors, and unusual or suspicious items or conditions are strictly for informational purposes.

Florida is underlain by a soluble limestone formation, which can dissolve and result in surface subsidence and the formation of sinkholes. A comprehensive assessment of the potential for sinkhole development at this site was not included in our scope of services. Typically, such an assessment would include geophysical surveying methods and the extension of deeper soil borings into the underlying limestone formation.

The recommendations submitted are based on the subsurface information obtained by PSI and available design information furnished by Place Maker Design Studio, LLC for the proposed project. If there are any revisions to the plans for this project or if deviations from the subsurface conditions noted in this report are encountered during construction, PSI should be notified immediately to determine if changes in the recommendations are required.

This report has been prepared for the exclusive use of Place Maker Design Studio, LLC and its consultants for the specific application to the proposed City of Tampa Park to be located near the northwestern corner of North Hubert Avenue and West Woodlawn Avenue in Tampa, Florida.

APPENDIX



Reference: Aerial obtained from Google

LEGEND

- Approximate SPT boring location
- Approximate Hand Auger boring location

BORING LOCATION PLAN

0 60'

PLAN SCALE

intertek psi

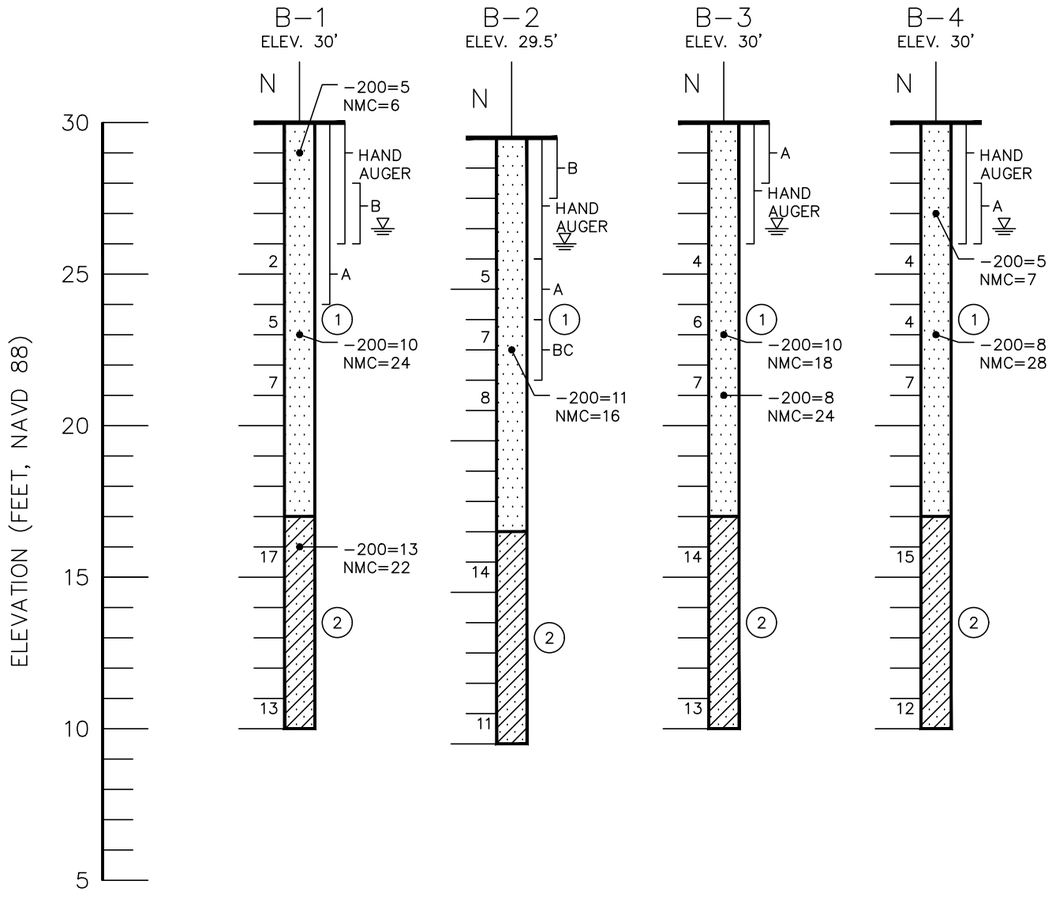
DRAWN	DJG
CHECKED	CA
APPROVED	LTL
SCALE	NOTED

GEOTECHNICAL SERVICES

PROPOSED CITY OF TAMPA PARK

TAMPA, FLORIDA

DATE	PROJ. NO.	SHEET
NOV 24	07753740	SHEET 1



SOIL PROFILES

0 5'

VERTICAL SCALE

LEGEND

①	Light brown/brown SAND to slightly silty SAND (SP/SP–SM)
②	Brown clayey SAND (SC)
A	With trace shell
B	With roots
C	With strong petroleum odors
SP	Unified Soil Classification System (ASTM D 2487) group symbol as determined by visual review
▽	Groundwater level, November 2024
GNE	Groundwater level not encountered
N	SPT N–value in blows/foot
–200	Fines passing No. 200 sieve (%)
NMC	Natural Moisture Content (%)

intertek psi

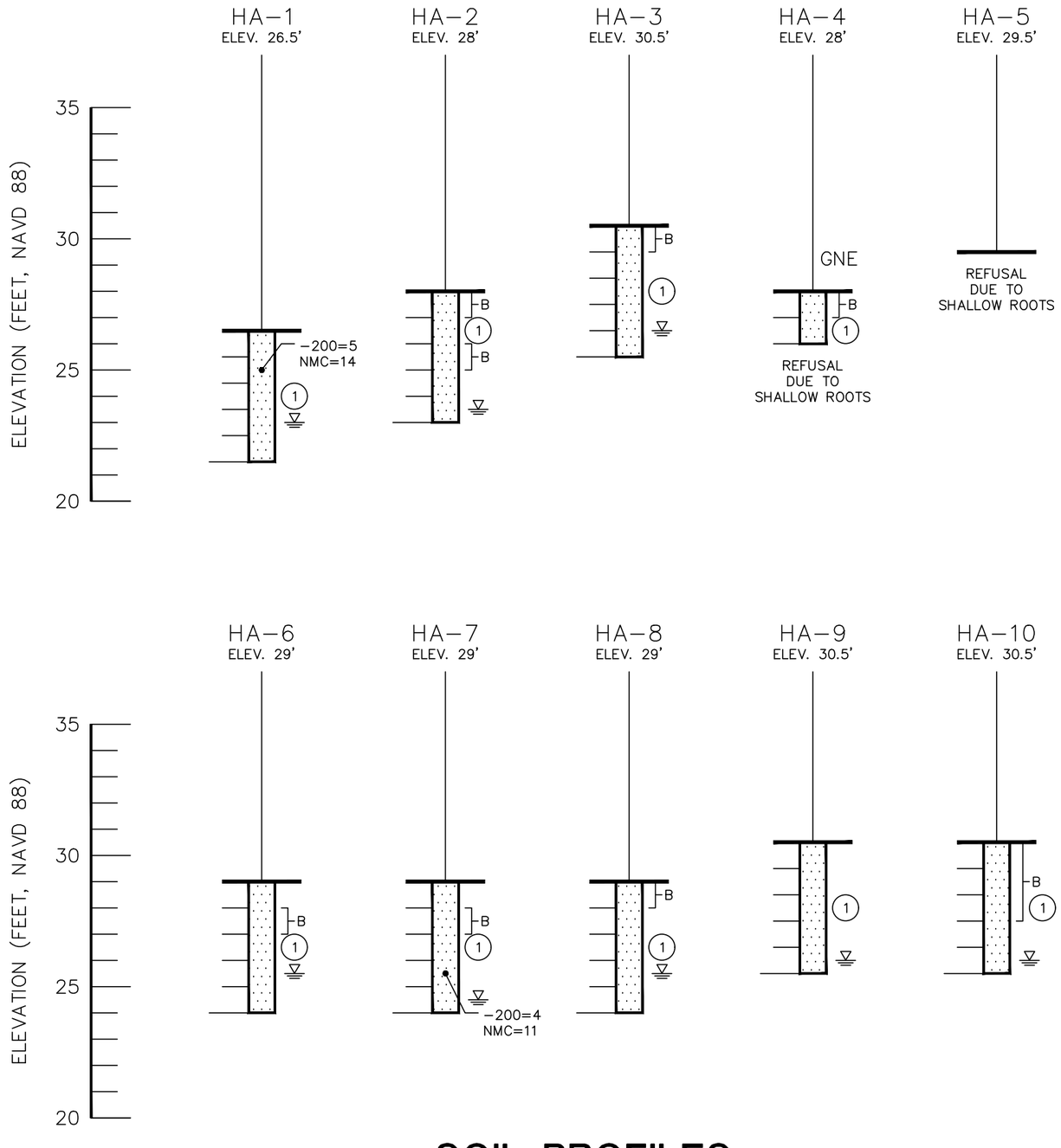
DRAWN	DJG
CHECKED	CA
APPROVED	LTL
SCALE	NOTED

GEOTECHNICAL SERVICES

PROPOSED CITY OF TAMPA PARK

TAMPA, FLORIDA

DATE	PROJ. NO.	SHEET
NOV 24	07753740	SHEET 2



SOIL PROFILES

0 5'

VERTICAL SCALE

LEGEND

1	Light brown/brown SAND to slightly silty SAND (SP/SP–SM)
2	Brown clayey SAND (SC)
A	With trace shell
B	With roots
C	With strong petroleum odors
SP	Unified Soil Classification System (ASTM D 2487) group symbol as determined by visual review
▽	Groundwater level, November 2024
GNE	Groundwater level not encountered
N	SPT N–value in blows/foot
-200	Fines passing No. 200 sieve (%)
NMC	Natural Moisture Content (%)

intertek psi

DRAWN	DJG
CHECKED	CA
APPROVED	LTL
SCALE	NOTED

GEOTECHNICAL SERVICES

PROPOSED CITY OF TAMPA PARK

TAMPA, FLORIDA

DATE	PROJ. NO.	SHEET
NOV 24	07753740	SHEET 3